

Experimental Andtheoric Approach to Slanted Concrete Cylinders Subjected to Uniaxial Pressure

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Abstract: In this study, one of the parameters which influence concrete compressive strength was investigated by using 50 standard high strength concrete cylinder specimens. This effect is inclination at one-end. When concrete filled molds are put on a sloping place, fresh-mixed concrete starts flow and inclination emerges after hardening. It is observed that failure loads are related with inclination. Angled cracks occurred in inclined specimens. Experimental behavior was presented by an exponential curve.

Key words: Slanted cylinders • Standard cylinder specimen • Compression strength • Concrete

INTRODUCTION

Standard cylinder specimens, $d=150\text{mm}$ and $h=300\text{mm}$, are used world-wide to determine concrete compressive strength by means of uniaxial compression tests. Concrete must be allowed to harden about twenty-four hours in molds and cured for 28 days. At the 28th day, specimens are tested in order to determine compression strength. It is the most widely-used mechanical property in projects when concrete is the load-bearing material. There are some factors affecting concrete's compressive strength, such as the type of cement, water-to-cement ratio, rate of loading, addition of cementitious materials, aggregate, moisture, temperature conditions during cure, age of concrete and capping. If cylinder specimen taken from the same concrete mix were exposed to different curing conditions, their strength values would become different [1, 2]. Under variable-rated loading, sometimes more than one peak points occur at stress-strain diagrams. High speed loading causes larger compression strength measurements [3]. Capping is another effect on compressive strength [4]. In order to minimize the variations, specimens must be carefully-treated to testing, such as the same curing conditions and the same loading rate. But there can be still variations between the strength values of the same-treated specimens. Distribution of aggregates in concrete mix, cavities in matrix or disorder in geometry should be considered besides many influencing factors. In this

regard, a type of geometric-disorder is investigated in this study. Mentioned disorder is inclination at one-end of the cylinder.

In practice, properly lubricated cylinder molds are filled in 3 levels with compaction. Fully stuffed molds are put in a suitable place to harden and unraveled after twenty-four hours [5,6]. Studied problem emerges at this waiting period. When molds are put on a sloping surface, wet concrete starts flow to be parallel where they are placed. Emergence of inclination can be seen in Fig. 1.

At the closed-end of mold, concrete has no inclination. Depending on compaction, it has a smooth surface. Concrete flows and hardens at the open-end of mold. One half rises and the other half descends. These elliptical surfaces are separated by a line which is 300 millimeters from the bottom and parallel to the bottom surface. This line is called in this study as "zero inclined line". It is denoted in figures as "s-s". Maximum and minimum height points from the bottom can be unified by a line. This line must be perpendicular to the zero line as to the geometry, Fig. 2.



Fig. 1: Emergence of inclination



Fig. 2: Maximum - minimum heighted points and zero line of cylinder

MATERIALS AND METHODS

Mixing Proportions of Concrete: Many aggregates can be used to produce high-strength concrete, such as the natural sand, granite, limestone, or dolomite. Although higher compression strength is generally achieved by using granite [7], the limestone aggregate was chosen due to the easy-found and common usage in Northern-Black Sea Region, Turkey. Aggregates were taken from the Macka/Meryemana region. The maximum aggregate size is 16 mm. Physical properties of aggregate are given in Table 1. CEM I 42,5 portland cement was used. The number 42.5 indicates its characteristic compressive strength in MPa. Properties of this cement is given in

Table 2. In addition to these, silica fume and superplasticiser (ASTM C-494 F type) were used in high performance concrete. Chemical properties of silica fume are given in Table 3. Mixing proportions of concretes are given in Table 4.

Inclination Measurements: Cylinders were placed on a horizontal and smooth surface. Zero lines, max.-min. directions and their downwards extensions were determined. Most inclination values were easily-visible and represented real cases. In this study, it is aimed to demonstrate the probable and overlooked circumstances. But extreme inclinations have also been evaluated, to make generalizations on this problem.

Table 1: Physical properties of aggregate

Aggregate size	Loose density (kg/m ³)	Dry density (kg/m ³)	Saturated density (kg/m ³)	Water absorption (%)
Coarse(>4mm)	1445	2706	2720	0,43
Fine(<4mm)	1485	2675	2682	0,50

Table 2: Properties of CEM I 42,5R portland cement

Physical properties				Mechanical properties	
Density,(g/cm ³)	Age (day)	Flexural strength (MPa)	Compressive strength (MPa)		
3.10	2	5.74	29.02		
Specific surface (Blaine) cm ² /g	2	5.74	29.02		
Setting time (vicat)	Initial (hours)	7	43.69		
	Final (hours)	28	52.92		
			8.74		

Table 3: Chemical properties of silica fume

Component (%)	SiO ₂	Fe ₂ O ₃	Al ₂ O ₃	CaO ₃	MgO ₃	CrO ₃	Loss on ignition	Free CaO
	82	1.8	3.2	1.4	5	3	2.2	1.2

Table 4: Mixing proportions of concrete

Concrete	W/C	Cement (kg/m ³)	Water (kg/m ³)	Total aggregate (kg/m ³)	Absorbed water (kg/m ³)	Admixtures	
						SP (kg/m ³)	SF (kg/m ³)
HPC	0.30	500	150	1789	4.2	22	50

HPC: high performance concrete,

SP : superplasticizer admixture, SF: silica fume

RESULTS AND DISCUSSION

Six specimens with minimum inclinations were tested to obtain mechanical properties. Because of the ease-of-use and common-usage, all of the cylinders were capped by having used sulphur mortar from both-ends. Tests were carried out for all cylinders when the concrete was 28-days-old. One of these specimens is shown in Fig. 3 with strain-gages. Average stress-strain curves are given in Fig.4 for high strength concrete, Young Modulus (E_c) and Poisson's ratio (ν) were determined and given with other mechanical properties in Table 5.

Cylinders were tested to determine failure loads. Loading rate was the same in every specimen as 0,24 MPa/sec. Failure loads at corresponding inclinations are given in Fig. 5.

To observe strain variation through sections, six strain gages were placed on another specimen. It is shown on Fig. 6. Zero-line had horizontal strain gage. By counter-clockwise with 15° , A(s)-A(s) cross section, another horizontal gage was placed. Maximum-minimum section has two longitudinal strain gages. And finally, counter-clockwise 15° with maximum-minimum section, two strain gages were placed longitudinally. It can be noticed that, A(max)-A(min) cross section is also perpendicular to A(s)-A(s). This specimen has $0,476^\circ$ of inclination and high performance concrete.

Circumferential strains at different two sections, s-s and A(s)-A(s), are given in Fig.7. Strain values decrease far from zero line. Through downward direction of minimum heighted point, the least circumferential strain values are obtained at corresponding loads.

Fig.8 presents longitudinal strains of cylinder in specified sections. At the vertical line of maximum heighted point, more shortening is achieved. It is seen that under the maximum heighted point, greater strain values are achieved compared to the 15° placed section. But these variations are not greater than the ones of the circumferential strains.

Strain variations of three specimens with different inclinations are given in Figs.9, 10, 11, 12 and 13.

Strain capability decreases for bigger inclination values. This situation also can be seen in Fig.10 for longitudinal strain values under minimum heighted point.

Circumferential strain variations are given in Figs. 11, 12 and 13. For an inclined cylinder, biggest circumferential strains are achieved in the downwards direction of 300 mm heighted, zero line.

Due to the non-uniformly distributed stresses, concrete cylinders were not able to bear greater loads as much as lesser-inclined ones. Concrete reaches its maximum strain at lower loads. Some crack patterns can be seen in Fig. 14.

There are many influences on concrete compression strength, such as the specimen sizes, specimen shapes and placement directions [8-10]. When inclination occurs, both-ends of cylinder are no-more the same. However, the volume of cylinder is still the same. Inclined surface becomes ellipse. Bottom surface remains circular.

In this study it is maintained that there is a relation between inclination and failure load based on test data. This relation can be expressed best by an exponential line. If these values are linearly connected, the line intersects with the vertical axis at 5.35 degrees for high



Fig. 3: Strain-gage placements

Table 5: Mechanical properties of concrete

Concrete	Number of specimens	Average compressive strength (Mpa)	Standard Deviation (Mpa)	Coefficient of variation	Modulus of elasticity (GPa)	Poisson's Ratio
HPC	50	69.49	14.16	0.21	32.2	0.237

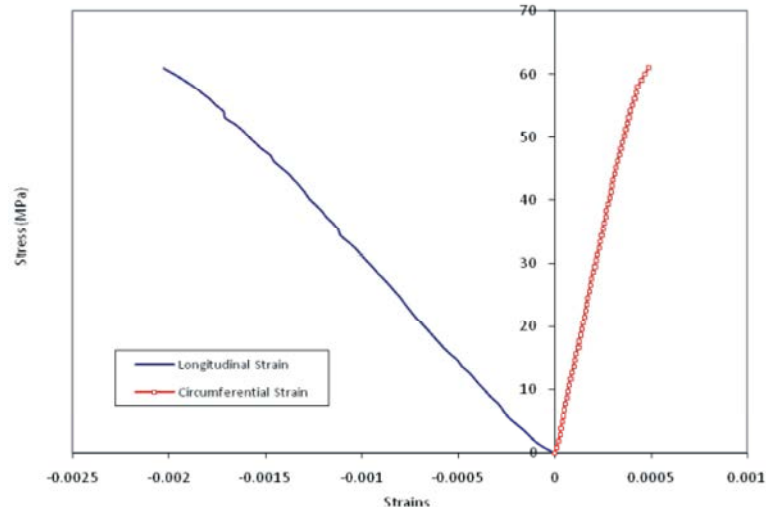


Fig. 4: Average stress-strain behavior of high performance concrete

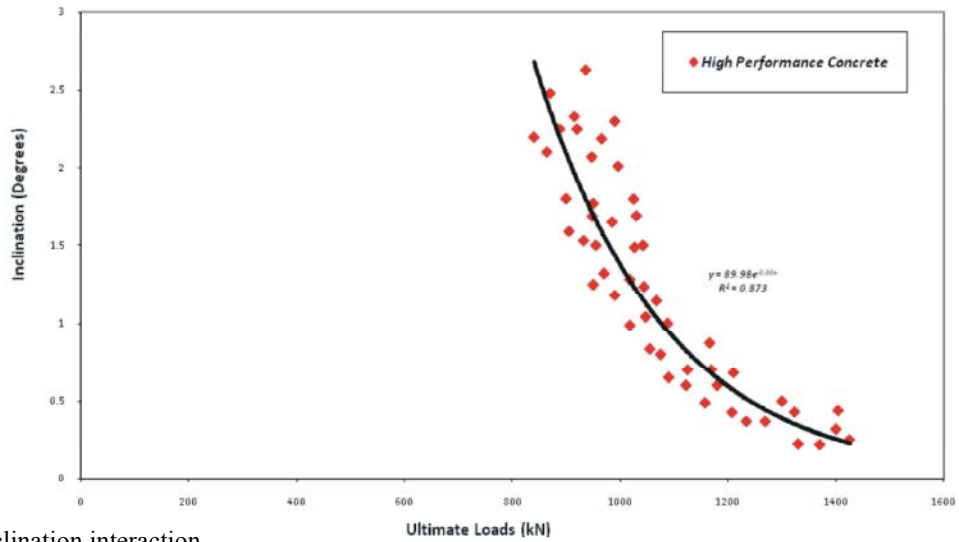


Fig. 5: Load-inclination interaction

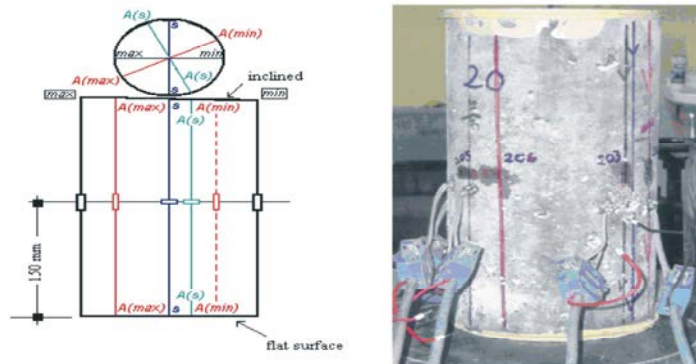


Fig. 6: View of strain-gage placements on a cylinder

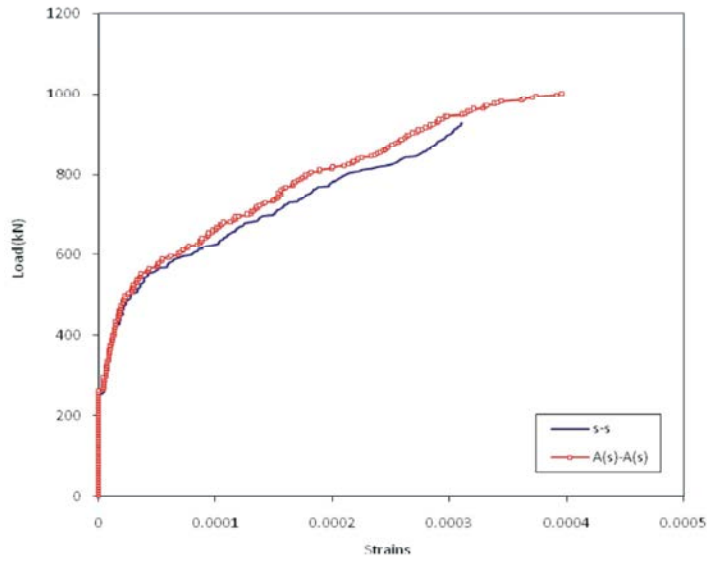


Fig. 7: Circumferential strains of cylinder

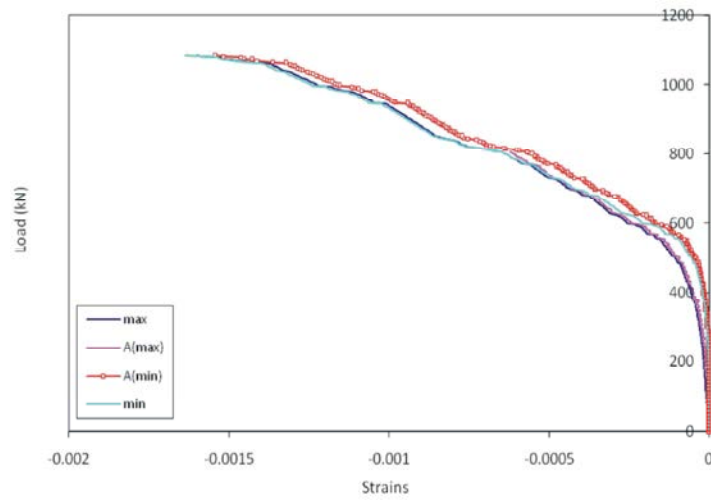


Fig. 8: Unit shortening (longitudinal strain) of cylinder

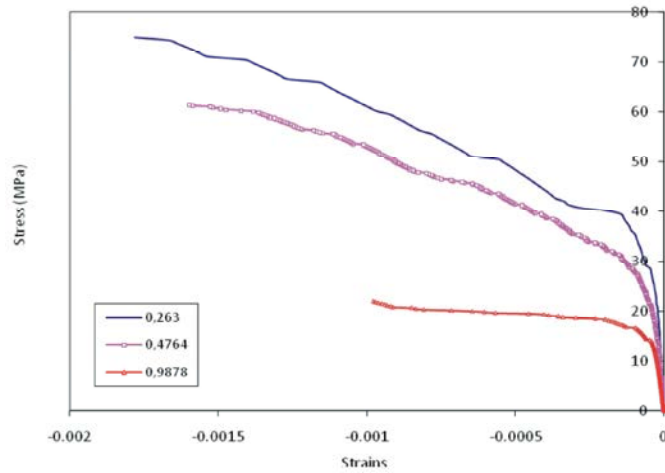


Fig. 9: Longitudinal strains under maximum heighted point

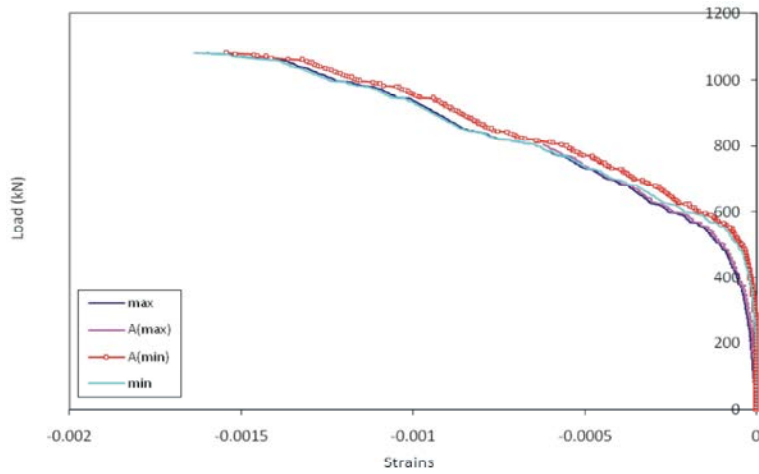


Fig. 10: Longitudinal strains under minimum heighted point

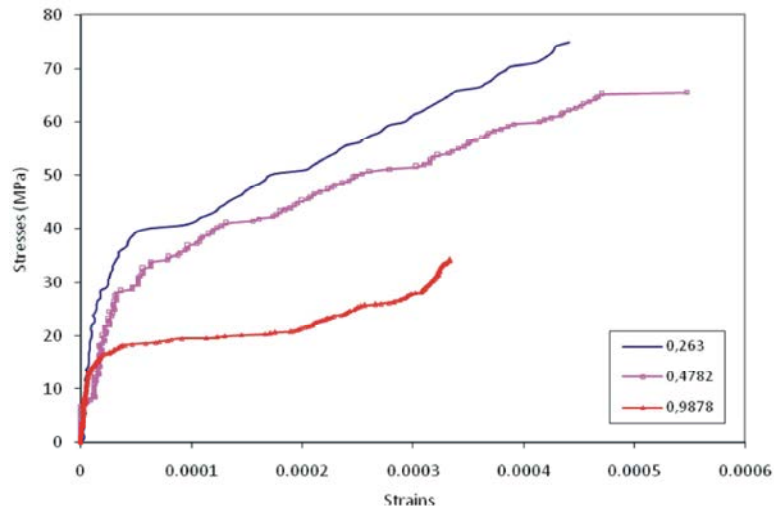


Fig. 11: Circumferential strains under maximum heighted point

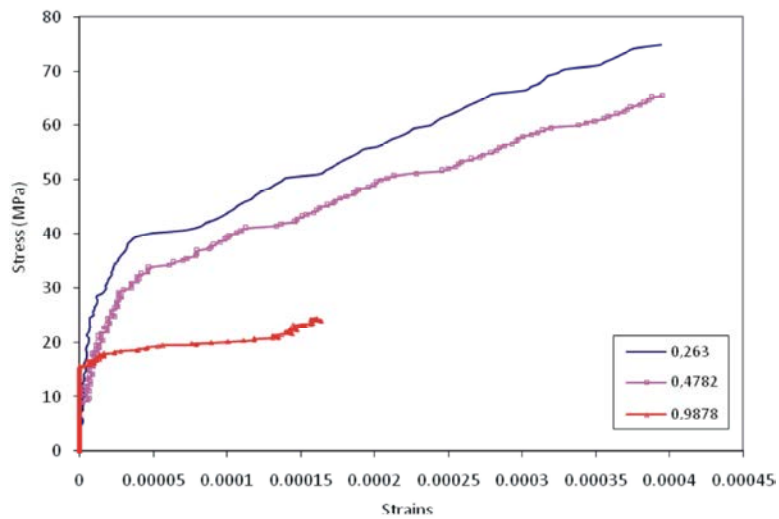


Fig. 12: Circumferential strain under minimum heighted point

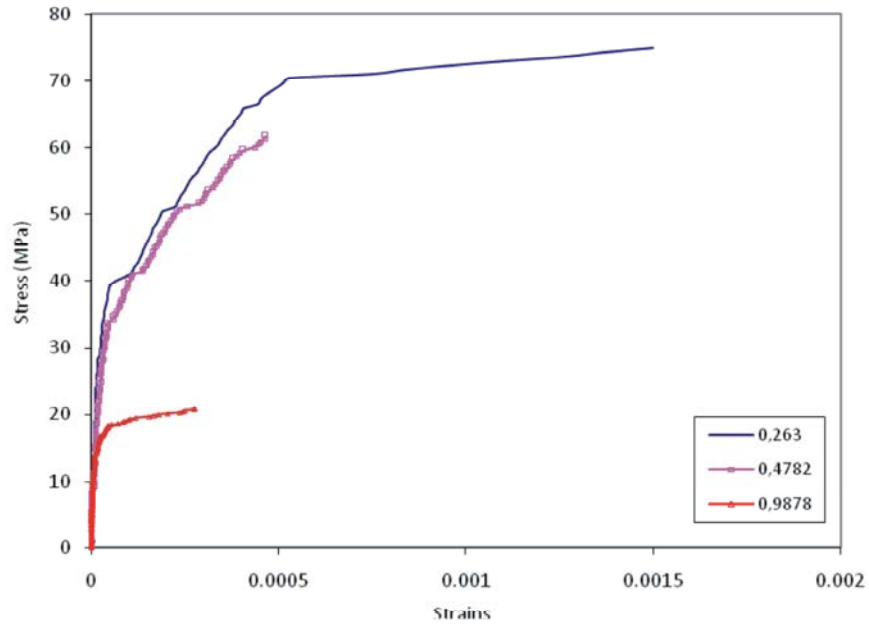


Fig. 13: Circumferential strains under 300 mm heightened line



Fig. 14: Crack patterns of some specimens

performance concrete, which represents maximum inclination. This means that zero-strength matches a cylindrical shape, which has maximum height of 307.2 mm and minimum height of 292.8 mm. A specimen with these geometric properties must have load-bearing capacity. Similar problems emerge when polynomial or logarithmic relations are used, Fig 15.

Exponential function is the most appropriate solution. Because, the inclination of cylinder, expressed as α , is limited to 90 degrees. When inclination is step by step increased to 90°, infinite long strip with an infinitesimal thickness emerges. This theoretic body has zero strength. At very near 90°, like 89,99°, specimen must have extremely- long height, very small thickness and nearly

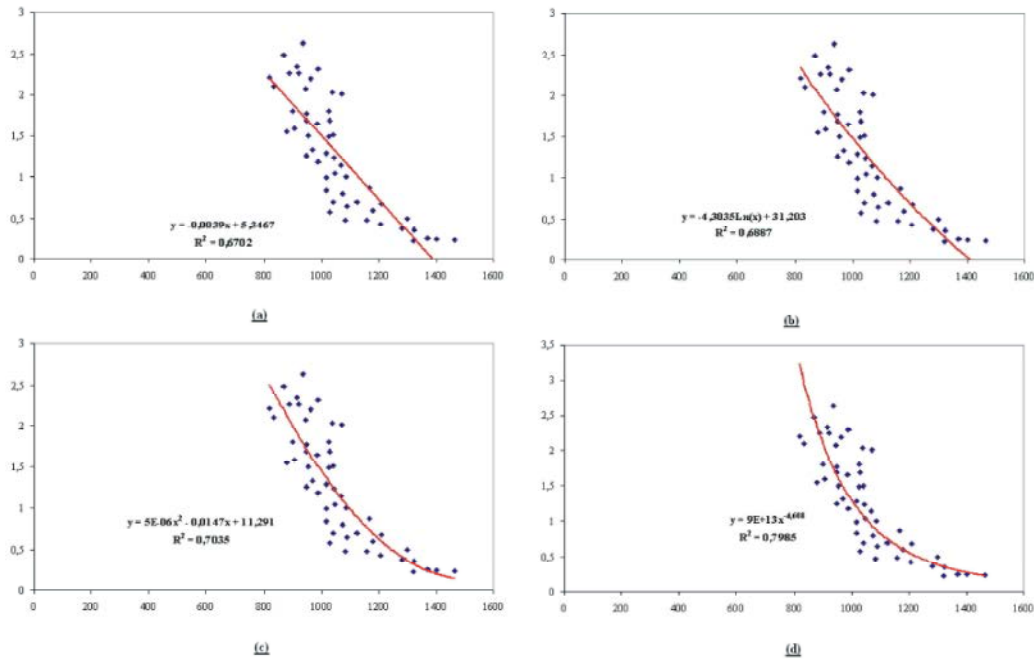


Fig. 15: Relating Failure Load to Inclination by Different Ways

zero load-bearing capacity. Average-relation between inclination and failure load is shown in Fig. 6 for high performance concrete as $y=89.980e^{-0.0042x}$. On the other hand, by the failure load point-of-view, line cannot intersect directly with any point in horizontal axis. Because, every cylinder has some amount of inclination whether too small or big.

CONCLUSIONS

Conclusions obtained from this study are given below.

There is no a perfect-shaped cylinder, namely, every concrete cylinder has some amount of inclination. Inclination values must be kept minimal in order to prevent variations in compression strength. In an inclined cylinder, very different compressive strength measurements could be obtained for the same concrete mix. There are also variations in Poisson's Ratio and Young Modulus.

When concrete cylinder specimens are tested under uniaxial load, similar strain distributions occur. And similar relation could be build by this way for lower concrete qualities by using the relation presented in this study. Curve would be still tangent to vertical axis at a point according to units. General form of relation is $\alpha=\alpha_{max}e^{-cf}$, where "c" denotes material coefficient ; "f" is failure load

and α_{max} is maximum inclination. When unit is degree for inclination, α_{max} equals to 90. If inclination is represented in radians, α_{max} becomes $\pi/2$ and the relation becomes $\alpha=(\pi/2)e^{-cf}$.

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